

Project No.: BRM-21000267-C0 March 22, 2021

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RE: Slope Assessment
Proposed Residential Subdivision
5 - 65 Archerhill Court, Aurora, Ontario

This letter report presents the results of a slope assessment of the existing slopes at 5 - 65 Archerhill Court, Aurora, Ontario. The subject site is located on the northwest quadrant of the intersection of Bayview Avenue and Vandorf Sideroad. Currently, the subject lands are divided into fourteen (14) residential lots, each developed with an occupied home surrounded by landscaped areas and scattered trees. It is understood that a residential subdivision is proposed for future development. In general, this site is bounded by Vandorf Sideroad to the south, Holland River Valley Trail to the west, a vacant parcel of land to the north, and Bayview Avenue to the east. It is understood that the west, north and northeast sides of the proposed development are close to regulated areas under the jurisdiction of the Lake Simcoe Region Conservation Authority (LSRCA), as illustrated in the map included in Appendix A.

The following documents were reviewed in conjunction with the provided drawings:

- Topographic Survey. R-PE Surveying Limited, Ontario Land Surveyors. CAD File No. 20298TP02, Job No. 20-298.
- Preliminary Concept Plan. Part of Lot 76, Concession 1, EYS, Town of Aurora. miCAD Inc. Project No. 20049, Drawing No. 04, dated December 30, 2020.
- Preliminary Geotechnical Investigation, Residential Subdivision Development, 5 to 65
  Archerhill Court, Aurora, Ontario. EXP Services Inc. (EXP) Project No. BRM-21000267A0, dated January 22, 2021.

The preliminary geotechnical investigation by EXP was carried out within the footprint of the proposed residential development. The preliminary concept plan and topographic map are

shown in Appendix B. The borehole location plan and borehole logs of the geotechnical investigation are presented in Appendix C.

#### **Visual Examination**

Visual examination of the existing slopes at the subject site was undertaken on March 10, 2021. The site observations were supplemented with survey information and relevant borehole data. The photographs showing the general conditions of the existing slopes taken during the visual examination are presented in Appendix D.

Our observations and findings during the site examination are summarized as follows:

#### Slope configuration, including location, height and inclination

North boundary: The slope at the north boundary of this site is a northward slope. The slope is up to about 8 m in height and the slope gradient varies from approximate 12 horizontal to 1 vertical (12H:1V) to 5.5H:1V.

South boundary: Along the south boundary of the site, the southward slope slopes down towards Vandorf Sideroad. The slope is up to about 2 m in height and the slope gradient is approximate 3.5H:1V.

West boundary: Near the west boundary, the ground slopes westward towards Holland River Valley Trail and a tributary of the Holland River. The slope is about 5 to 10 m in height and the slope gradient varies from approximate 9H:1V to 4H:1V. A local slope with a gradient of about 3H:1V with a height of 6 m was observed near the intersection of Vandorf Sideroad and Holland River Valley Trail.

East boundary: The slope at the east boundary of this site is an eastward slope. The slope is up to about 7 m in height and the slope gradient varies from approximate 6H:1V to 5.5H:1V.

Slopes towards Archerhill Court: The slopes from the existing buildings towards Archerhill Court are up to about 4 m in height, and approximate 5H:1V or flatter.

The slope near the west project limit is considered to be critical due to the slope configurations of its maximum height and having the steepest slope gradient.

#### Structures in the vicinity of the slopes

In general, the distribution of the existing residential buildings is in a horseshoe shape around Archerhill Court. The buildings were observed near or at the physical top of slope. Archerhill Court slopes downward from south to north.



#### Signs of previous landslide, erosion, and other unstable features

No surficial longitudinal cracking was observed on the slope crests. No sign of previous shallow or deep-seated landslide on the existing slope faces along or perpendicular to the physical top of slopes was discovered during the visual examination. No other tension crack or erosion was observed on the slope faces. No erosion was observed at the slope toes near the tributaries of the Holland River.

#### Soil stratigraphy in exposed slope faces

Soil stratum exposure was observed near the tributary of the Holland River near the west boundary during the visual examination. Moist to wet clayey soils with some sand content were found at the slope toe near the intersection of Vandorf Sideroad and Holland River Valley Trail. No soil stratum exposure was observed at the other areas.

#### Surface drainage and seepage zone

Based on the slope configurations, the surface drainage from this site is anticipated to be discharged to the nearby roadways or open spaces. No seepage zone was identified on any existing slope crest, face and toe.

Within the project limits, the surface water is collected by the ditches along Archerhill Court, and discharged through the storm outlet between 65 and 70 Archerhill Court towards the open space and a tributary of the Holland River beyond the north project limit.

#### Vegetative cover

The slopes are covered with matured trees and small vegetation such as shrubs and weeds. The majority of the tree trunks are near vertical.

#### Watercourse features

Tributaries of the northward flowing Holland River were observed at the slope toes on the west, north and northeast sides of the proposed development. During the visual examination, the bankful width of the stream near the west boundary was about 2 to 3 m. The water depth of the stream was less than 0.4 m at the time of the investigation.

#### Proposed development

The proposed residential subdivision includes 146 residential lots for the future development. Based on the concept plan, the footprint of the proposed lots occupies the majority of the site, and open spaces were kept on the west, north and northeast sides of the subject site for the regulated area of the LSRCA. The distance from the proposed residential lots to the tributaries of Holland River is from about 50 to 150 m.



## **Subsurface Condition Summary**

In the previous geotechnical investigation, a total of seven (7) boreholes (designated as Boreholes 1 through 7) were drilled to a depth of 8.2 m below existing grade. The approximate borehole locations and logs are shown in Appendix C.

Reference is made to the aforementioned geotechnical report by EXP for detailed soil profiles encountered on site. The following is a brief description of the subsurface conditions encountered during the previous investigation within the study area.

#### **Soil Conditions**

#### Topsoil

Topsoil was encountered from ground surface in all boreholes. The thicknesses of topsoil ranged from 125 to 225 mm.

#### **Fills**

Dark brown to brown cohesive fills / reworked parent soils were encountered below the topsoil in Boreholes 1, 3, 6 and 7. The fills were found to extend to elevations ranging from 276.8 to 268.5 m. It should be noted that a layer of topsoil with a thickness of 0.7 m was observed below the fills in Borehole 1; and in Borehole 6, the cohesive fills are overlain by a layer of 0.1 m thick non-cohesive fills.

With the recorded SPT N-values ranging from 3 to 12 blows/0.3 m, the fills were in a typical loose state of compactness. The fills have measured moisture contents from 16 to 32 percent of dry weight indicating a moist condition. The high moisture contents are likely due to the presence of organic matters. The measured unit weight of the fills was from 18.8 to 21.5 kN/m<sup>3</sup>.

#### Silty Clay

A layer of silty clay was encountered below the fills in Boreholes 1, 3, 6 and 7, and encountered below the topsoil in the remaining boreholes. This soil unit extended to the borehole termination elevations from 269.7 to 263.8 m.

This silty clay deposit is brown to grey in colour and has a typical stiff to very stiff consistency, with the SPT N-values ranging from 3 to in excess of 50 blows/0.3 m. The moisture contents within this deposit were found to range from 17 to 28 percent of dry weight, indicating generally a moist to wet condition. The measured unit weight of the representative samples ranged from 19.0 to 21.8 kN/m<sup>3</sup>.



#### **Groundwater Conditions**

Groundwater levels were observed in the open boreholes during the course of the fieldwork. Water level monitoring was also carried out in the installed monitoring wells in the previous geotechnical investigation. Short-term observations in the monitoring wells are summarized in the following table.

Borehole No.	Depth/Elevation of Monitoring Well Tip (m)	Screened Strata	Date of Water Level Measurement (mm/dd/yyyy)	Measured Water Level Depth/Elevation (m)		
2	7.6 / 268.0	Silty clay	01/20/2021	2.2 / 273.4		
5	7.6 / 270.4	Silty clay	01/20/2021	0.7 / 277.3		
6	7.6 / 268.7	Silty clay	01/20/2021	3.6 / 272.8		

Based on the locations and configurations of the monitoring wells, and the observation of groundwater at the time of the investigation, the groundwater level was envisaged to vary with ground surface topography, and towards the streams near the west, north and northeast project limits.

### Slope Stability, Toe Erosion Allowance and Setback

#### **Slope Stability**

Based on the results of visual examination and the topographic study, in relative terms, the slope near the west boundary is considered to be the steepest and therefore, selected for slope stability assessment. The maximum height of the slope in this area is about 10 m, and the slope gradient is 3H:1V or flatter. The minimum distance from the proposed residential lots to the nearby watercourse is greater than 50 m.

According to Watershed Development Guideline, LSRCA (June 1, 2020), stable slope allowance is determined to be 3H:1V. At the critical location of the southwest corner of this site, as the maximum slope height is about 7 m, the stable top of slope is estimated to be 21 m from the slope toe / stream, with an imaginary slope of 3H:1V towards the Holland River Valley Trail.

#### **Toe Erosion Allowance and LSRCA Required Setback**

The regression of the slope toe, due to erosion, over the design life of a typical structure (typically 100 years for long-term) is compensated in the Ontario Ministry of Natural Resources (MNR) Policy Guidelines by the introduction of a toe erosion allowance which is measured as a horizontal distance from the existing river / stream bank. The toe erosion allowance is based on the type of the slope toe material and the stream characteristics including the distance between the stream edge and the slope toe, bankfull width as well as the current toe erosion condition. A



toe erosion allowance is required in areas where the watercourse position is within 15 m of the slope toe.

Near the west boundary of the proposed development, especially at the southwest corner, the distance from the existing slope toe to the stream is less than 15 m. Therefore, in general, a toe erosion allowance should be considered. For cohesive soils where no active erosion is evidenced, the MNR Policy Guidelines recommends a minimum toe erosion allowance of 1 m for a stream with bankfull width less than 5 m.

According to Watershed Development Guideline, LSRCA (June 1, 2020), all development shall be setback the greater value of either a minimum of 15 m from the stable slope line or top of bank.

#### Conclusion

Based on the afore-mentioned stable slope allowance, toe erosion allowance and LSRCA required setback, the total setback from the existing stream is tabulated in the following table.

Maximum Distance of Stable Top of Slope	Toe Erosion Allowance	LSRCA Required Setback	Total Setback from the Existing Stream			
(m)	(m)	(m)	(m)			
30	1	15	46			

Based on the tabulated values, the maximum total setback is about 46 m from the existing stream. This value is less than the minimum distance from the proposed residential lots to the stream, i.e. 50 m. Therefore, the proposed concept plan meets the LSRCA requirements for watershed development.

#### Closure

The comments and recommendations are based on our understanding of the design features of the proposed development. If actual features vary or changes are made, we should review the changes in order to modify our recommendations, as required.



We trust that this letter meets your immediate requirements. Should you have any queries or if you require any further assistance, please do not hesitate to contact this office.

Yours truly,

**EXP Services Inc.** 

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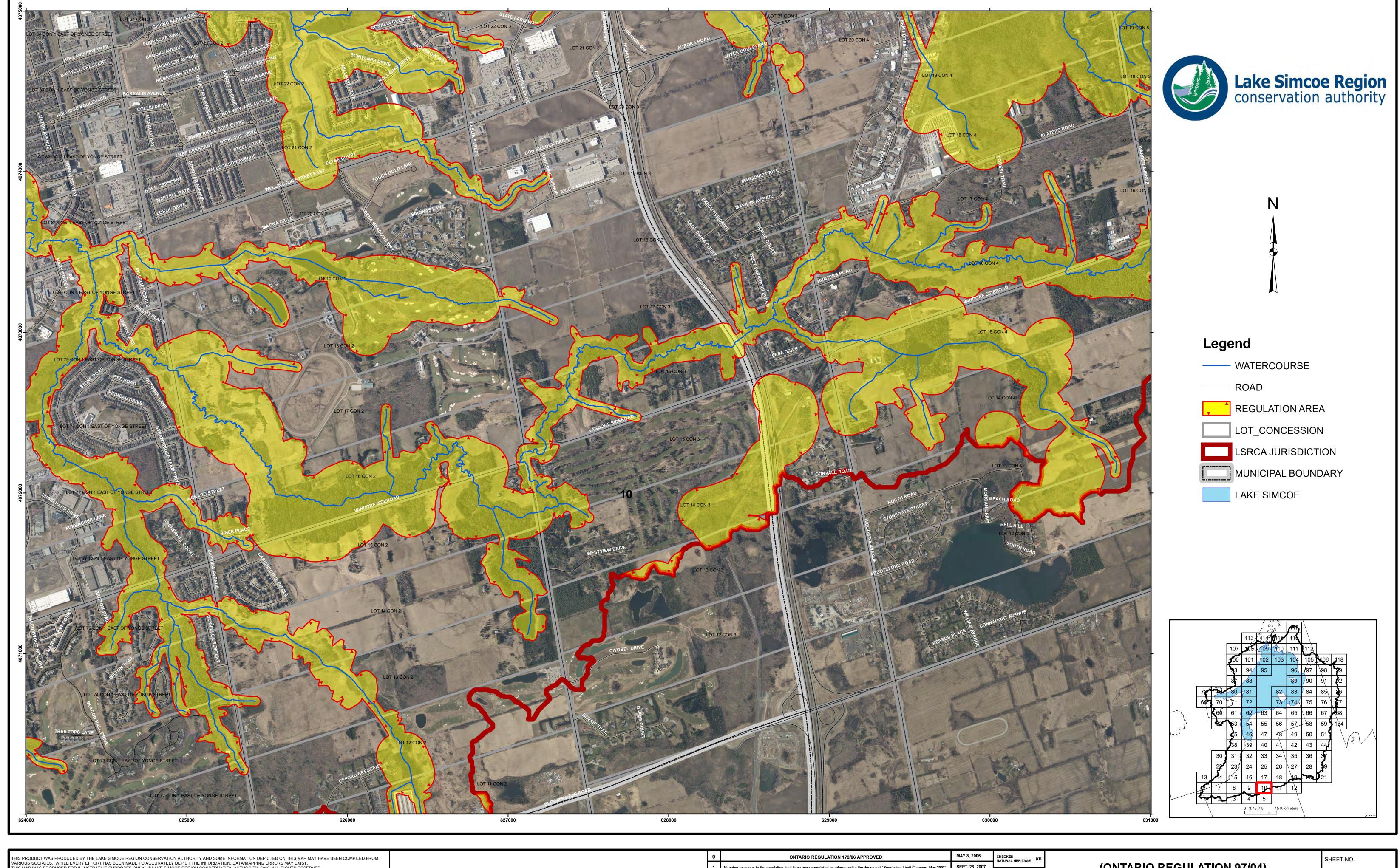
# **APPENDICES**



### **APPENDIX A**

LSRCA. Regulation of Development, Interference with Wetlands and Alterations to Shorelines and Watercourses. Sheet No. 10



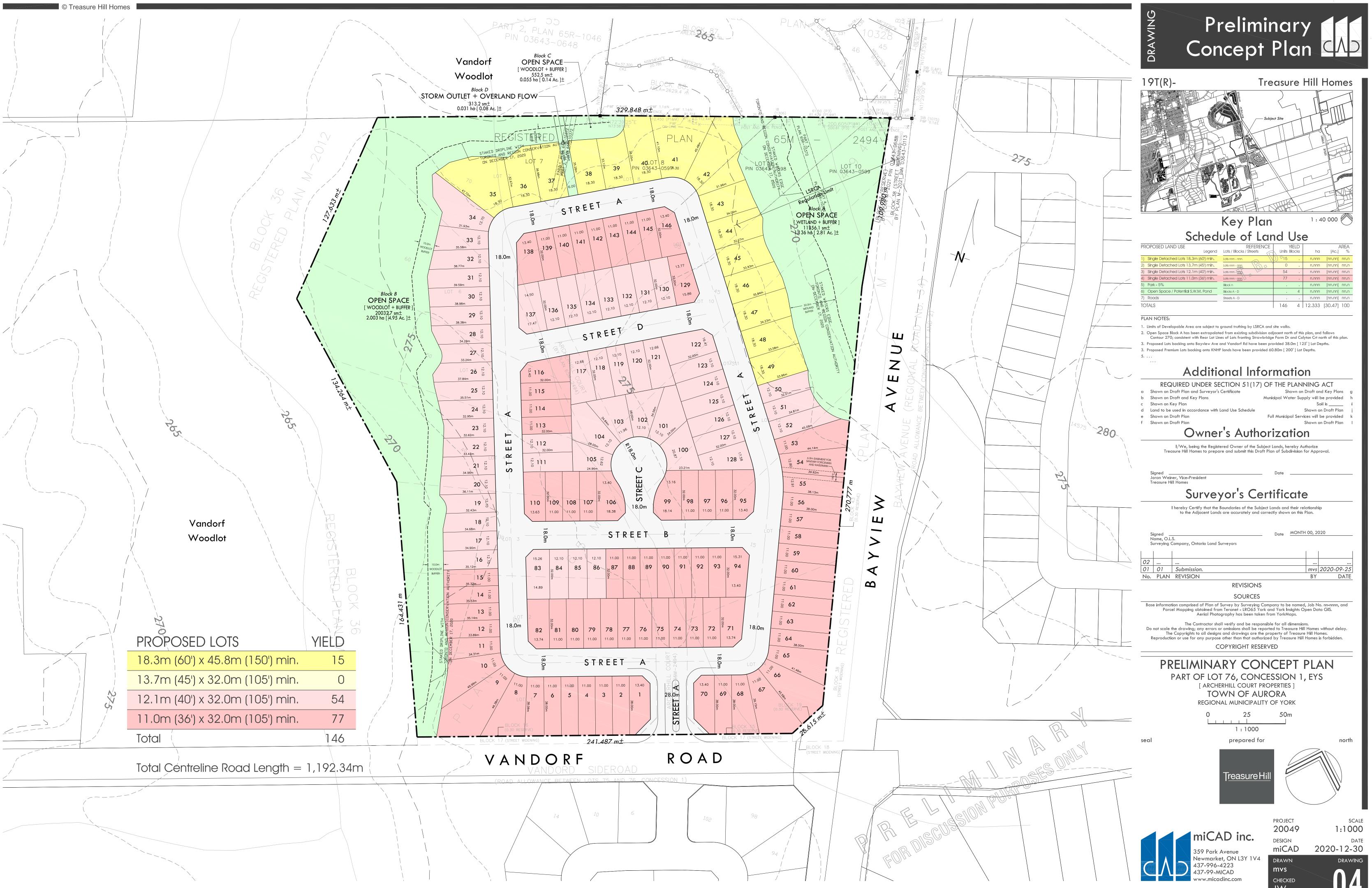


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VARIOUS SOURCES. WHILE EVERY EFFORT HAS BEEN MADE TO ACCURATELY DEPICT THE INFORMATION, DATA/MAPPING ERRORS MAY EXIST. THIS MAP WAS PRODUCED FOR ILLUSTRATIVE PURPOSES ONLY. © LAKE SIMCOE REGION CONSERVATION AUTHORITY, 2019. ALL RIGHTS RESERVED.				1 Mapping revisions to the regulati	on limit have been completed as referenced in the document "Regulation Limit Changes, May 2007"		OUTOWED NEWTOOL	(ONTARIO REGULATION 97/04)  JP  REGULATION OF DEVELOPMENT, INTERFERENCE WITH WETLANDS AND ALTERATIONS TO SHORELINES AND WATERCOURSES.  ONTARIO REGULATION 179/06			
THE FOLLOWING DATA SETS OF MUNICIPAL BOUNDARY, LOT_CONCESSION ARE © QUEEN'S PRINTER FOR ONTARIO (2018). REPRODUCED WITH PERMISSION.  ORTHOPHOTOGRAPHY 2018, 2016, 2013, 2008 © FIRST BASE SOLUTIONS INC.		1:10,000	2 Mapping revisions to the regulation Ortho imagery changed to reflect	n limit have been completed as referenced in the document "Regulation Limit Changes, April 2009". nost recent imagery available. Map/data disclaimer changed to reflect most current disclaimers in use	APRIL 24, 2009	REGULATIONS JF					
RIVERINE HAZARDS WERE BASED ON EXISTING FLOOD PLAIN MAPPING. FLOOD PLAIN LIMITS WHERE ENGINEERING PRODUCTS DID NOT EXIST WERE DETERMINED BY LSRCA STAFF. RIVERINE EROSION HAZARDS WERE DETERMINED BY LSRCA STAFF. A 15-METER SETBACK WAS APPLIED FROM THE LIMITS OF ALL RIVERINE HAZARDS.		1110,000		3 Mapping revisions to the regulation Ortho imagery changed to reflect	n limit have been completed as referenced in the document "Regulation Limit Changes, July 2013". nost recent imagery available. Map/data disclaimer changed to reflect most current disclaimers in use	JULY 26, 2013	CHECKED - JF ENGINEERING JF			·	10
SHORELINE FLOOD HAZARDS WERE DETERMINED BY LSRCA STAFF BY APPLYING THE EQUATIONS PREVIOUSLY DEVELOPED THROUGH AN ENGINEERING STUDY. SHORELINE EROSION HAZARDS WERE DETERMINED BY LSRCA STAFF.	0 0.225 0.45	0.9	1.35 1.8	4 Mapping revisions to the regulation limit have been completed as referenced in the document "Regulation Limit Changes, September, 2014". Ortho imagery reflects most recent imagery available. Map/data disclaimer changed to reflect most current information.		SEPT. 26, 2014	APPROVED TH				
WETLANDS WERE DELINEATED BY THE MINISTRY OF NATURAL RESOURCES & FORESTRY. SETBACKS OF 120-M FROM PROVINCIALLY SIGNIFICANT WETLANDS (PSWs) AND 30-M FROM ALL OTHER WETLANDS WERE APPLIED.  MEANDERBELT WIDTHS WERE ESTABLISHED THROUGH A STUDY IN THE ABSENCE OF A STUDY MEANDERBELT WIDTHS WERE CALCULATED AS 20 TIMES THE BANKEULL WIDTHS.		Kilometers		n limit have been completed as referenced in the document "Regulation Limit Changes, April, 2018". t imagery available. Map/data disclaimer changed to reflect most current information.	APRIL 1, 2018	DATE: JANUARY 2006					
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PLEASE REFER TO "REFERENCE MANUAL FOR DETERMINATION OF REGULATION LIMITS" (LSRCA, 2005) OR CONTACT LSRCA (905-895-1281) FOR MORE INFORMATION				NO.	REVISIONS	DATE	WIMPPLOBI. JE	FILE EOCATION.		J OF	107

## **APPENDIX B**

**Preliminary Concept Plan and Topographic Map** 







# APPENDIX C Borehole Location Plan and Borehole Logs



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BRM-21000267-A0 Project No. Drawing No. Geotechnical Investigation Sheet No. 1 of 1 Project: Archerhill Court, Aurora Location: Combustible Vapour Reading  $\boxtimes$ Auger Sample January 8, 2021 Natural Moisture X Date Drilled:  $O \square$ SPT (N) Value Plastic and Liquid Limit CME 75 Track Drill Type: Dynamic Cone Test Undrained Triaxial at  $\oplus$ Shelby Tube % Strain at Failure Geodetic Datum: Field Vane Test Combustible Vapour Reading (ppm) SPT (N Value) Natural Soil/Rock Symbol ELEV. Unit Weight kN/m<sup>3</sup> Natural Moisture Content % Atterberg Limits (% Dry Weight) Soil Description Shear Strength 272.06 ~ 175 mm TOPSOIL over ~271.9 FILL: silty clay, trace sand, brown, -moist (reworked parent material) 20.9 ď 20.8 ô 20.7 19.8 19.5 ~268.5 TOPSOIL Ô 18.8 ~267.8 SILTY CLAY: trace sand, silt partings, brown, moist, stiff  $\overset{14}{\circ}$ 21.5 - Becoming grey, wet, hard 21.0 20.7 ~263.8 **END OF BOREHOLE** Hole Open to (m) **\***ехр. On Completion 3.05 Open

BRM-21000267-A0 Project No. Drawing No. Sheet No. 1 of 1 Geotechnical Investigation Project: Archerhill Court, Aurora Location: Combustible Vapour Reading  $\boxtimes$ Auger Sample January 8, 2021 Date Drilled: Natural Moisture X  $O \square$ SPT (N) Value Plastic and Liquid Limit CME 75 Track Dynamic Cone Test Drill Type: Undrained Triaxial at  $\oplus$ Shelby Tube % Strain at Failure Geodetic Datum: Field Vane Test Combustible Vapour Reading (ppm) SPT (N Value) Natural Soil/Rock Symbol ELEV. Unit Weight kN/m<sup>3</sup> Natural Moisture Content % Atterberg Limits (% Dry Weight) Soil Description Shear Strength 275.61 ~ 175 mm TOPSOIL over ~275.4 SILTY CLAY: trace sand, silt - partings, brown, moist, stiff to very stiff -20.5 19.7 20.4 0 19.8 - Becoming grey, wet, firm to stiff ö Ć 21.1 Ò Ö 20.1 ~267.4 **END OF BOREHOLE** Hole Open to (m) **\***ехр. On Completion Dry Open January 20, 2021 2.24

BRM-21000267-A0 Project No. Drawing No. Geotechnical Investigation Sheet No. 1 of 1 Project: Archerhill Court, Aurora Location: Combustible Vapour Reading  $\boxtimes$ Auger Sample January 8, 2021 Natural Moisture X Date Drilled: 0 🛮 SPT (N) Value Plastic and Liquid Limit CME 75 Track Dynamic Cone Test Drill Type: Undrained Triaxial at  $\oplus$ Shelby Tube % Strain at Failure Geodetic Datum: Field Vane Test Combustible Vapour Reading (ppm) SPT (N Value) Natural Soil/Rock Symbol ELEV. Unit Weight kN/m<sup>3</sup> Soil Description Natural Moisture Content % Atterberg Limits (% Dry Weight) Shear Strength 275.28 ~ 125 mm TOPSOIL over ~275.2 20.6 FILL: brown silty clay, trace sand, trace gravel, occasional rootlets, moist đ 21.5 ~273.1 SILTY CLAY: trace sand, silt partings, brown, moist, stiff 19.8 20.4  $\overset{13}{\text{O}}$ 20.2 - Becoming wet, firm 20.0 - Becoming grey, soft ô ~267.1 **END OF BOREHOLE** Hole Open Date to (m) **\***ехр.

BRM-21000267-A0 Project No. Drawing No. Geotechnical Investigation Sheet No. 1 of 1 Project: Archerhill Court, Aurora Location: Combustible Vapour Reading  $\boxtimes$ Auger Sample January 7, 2021 Date Drilled: Natural Moisture X  $O \square$ SPT (N) Value Plastic and Liquid Limit CME 75 Track Dynamic Cone Test Drill Type: Undrained Triaxial at  $\oplus$ Shelby Tube % Strain at Failure Geodetic Datum: Field Vane Test Combustible Vapour Reading (ppm) SPT (N Value) Natural Soil/Rock Symbol ELEV. Unit Weight kN/m<sup>3</sup> Natural Moisture Content % Atterberg Limits (% Dry Weight) Soil Description Shear Strength 275.63 ~ 125 mm TOPSOIL over 21.6 ~275.5 SILTY CLAY: trace sand, silt - partings, brown, moist, stiff to very stiff -X 8 20.4 8 21.8 × - Trace gravel O 20.5 20.8 - Becoming grey Ö 21.1 - Becoming wet - Becoming firm Ŏ 19.6 ~267.4 **END OF BOREHOLE** Hole Open to (m) **\***ехр. On Completion Dry Open

BRM-21000267-A0 Project No. Drawing No. Geotechnical Investigation \_1\_ of \_1\_ Project: Sheet No. Archerhill Court, Aurora Location: Combustible Vapour Reading  $\boxtimes$ Auger Sample January 7, 2021 Date Drilled: Natural Moisture X  $O \square$ SPT (N) Value Plastic and Liquid Limit CME 75 Track Drill Type: Dynamic Cone Test Undrained Triaxial at  $\oplus$ Shelby Tube % Strain at Failure Geodetic Datum: Field Vane Test Combustible Vapour Reading (ppm) SPT (N Value) Natural Soil/Rock Symbol ELEV. Unit Weight kN/m<sup>3</sup> Soil Description Natural Moisture Content % Atterberg Limits (% Dry Weight) Shear Strength 277.96 ~ 150 mm TOPSOIL over ~277.8 SILTY CLAY: trace sand, silt partings, brown, moist, stiff to hard 20.8 ď 20.8 21.3 20.0 20.4 Ö 20.4 19.9 - Becoming grey, wet, soft ~269.7 **END OF BOREHOLE** Hole Open Date to (m) **\***ехр. On Completion 4.27 Open January 20, 2021 0.67

BRM-21000267-A0 Project No. Drawing No. Geotechnical Investigation Sheet No. 1 of 1 Project: Archerhill Court, Aurora Location: Combustible Vapour Reading  $\boxtimes$ Auger Sample January 7, 2021 Natural Moisture X Date Drilled: 0 🛮 SPT (N) Value Plastic and Liquid Limit CME 75 Track Drill Type: Dynamic Cone Test Undrained Triaxial at  $\oplus$ Shelby Tube % Strain at Failure Geodetic Datum: Field Vane Test Combustible Vapour Reading (ppm) SPT (N Value) Natural Soil/Rock Symbol ELEV. Unit Weight kN/m<sup>3</sup> Soil Description Natural Moisture Content % Atterberg Limits (% Dry Weight) m Shear Strength 276.34 ~ 175 mm **TOPSOIL** over ~276.2 FILL: ~ 100 mm brown silty sand over-brown silty clay (reworked parent Ó 20.0 ~275.6 material), moist SILTY CLAY: trace sand, silt partings, brown, moist, stiff to very stiff 20.5 2.2 21.3 20.8 - Becoming grey, wet  $\overset{15}{\circ}$ 20.7 - Becoming firm 20.8 20.9 ~268.1 **END OF BOREHOLE** Hole Open Date Level to (m) **\***ехр. On Completion 7.01 Open January 20, 2021 3.55

BRM-21000267-A0 Project No. Drawing No. Geotechnical Investigation Sheet No. 1 of 1 Project: Archerhill Court, Aurora Location: Combustible Vapour Reading  $\boxtimes$ Auger Sample January 7, 2021 Date Drilled: Natural Moisture X 0 🛮 SPT (N) Value Plastic and Liquid Limit CME 75 Track Dynamic Cone Test Drill Type: Undrained Triaxial at  $\oplus$ Shelby Tube % Strain at Failure Geodetic Datum: Field Vane Test Combustible Vapour Reading (ppm) SPT (N Value) Natural Soil/Rock Symbol ELEV. Unit Weight kN/m<sup>3</sup> Natural Moisture Content % Atterberg Limits (% Dry Weight) Soil Description m 277.88 ~ 225 mm TOPSOIL over ~277.7 FILL: mix of silty clay and topsoil, brown to dark brown, moist Ж 20.0 Ő ~276.8 SILTY CLAY: trace sand, silt 19.0 partings, brown, moist stiff to hard ð 20.0 21.1 20.5 - Becoming grey, wet, firm ö 21.0 ~269.7 **END OF BOREHOLE** Hole Open to (m) **\***ехр. On Completion Dry Open

# APPENDIX D Site Photographs





Photograph #1: Archerhill Crt, North End, Looking South



Photograph #2: 30 Archerhill Crt, Looking Northeast





Photograph #3: 30 Archerhill Crt, Looking Southeast

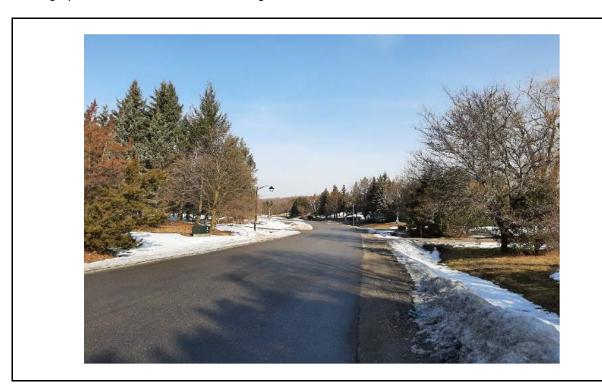


Photograph #4: 30 Archerhill Crt, Looking Southwest





Photograph #5: 30 Archerhill Crt, Looking Northwest



Photograph #6: Archerhill Crt & Vandorf Sideroad, Looking North





Photograph #7: Vandorf Sideroad & Holland River Valley Trail, Looking North



Photograph #8: Vandorf Sideroad & Holland River Valley Trail, Looking East





Photograph #9: Vandorf Sideroad & Holland River Valley Trail, Looking West



Photograph #10: Vandorf Sideroad & Holland River Valley Trail, Holland River, Looking North





Photograph #11: Holland River Valley Trail, Close to South End, Looking North



Photograph #12: Holland River Valley Trail, Close to South End, Looking Southwest





Photograph #13: Holland River Valley Trail, Middle, Looking Southwest



Photograph #14: Holland River Valley Trail, Middle, Looking North





Photograph #15: Holland River Valley Trail, Middle, Looking South



Photograph #16: Holland River Valley Trail, Middle, Looking Northwest





Photograph #17: Northwest Corner of Project Limits, Looking East



Photograph #18: Northwest Corner of Project Limits, Looking South





Photograph #19: North Project Limit, Middle, Looking North



Photograph #20: Northeast Corner of Project Limits, Looking Southwest





Photograph #21: Bayview Ave, Middle of East Project Limit, Looking West



Photograph #22: Bayview Ave, Middle of East Project Limit, Looking North





Photograph #23: Bayview Ave, Middle of East Project Limit, Looking South



Photograph #24: Bayview Ave & Vandorf Sideroad, Looking West

